ADAPTIVE NUMERICAL SIMULATION OF SEDIMENT TRANSPORT IN SHALLOW WATER FLOWS

F. Benkhaldoun*, S. Sahmim^x, M. Seaid⁺

* Université Paris 13 , *Ecole Polytechnique de Tunisie, Tunisia, ⁺School of Engineering, University of Durham

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Introduction

• Presentation of SRNH scheme

- Application to 1*D* Shallow Water flow and equilibrium propertie
- Application of *SRNH* scheme to pollutant transport in the strait of Gibraltar
- Application of *SRNH* scheme to a moving bed problem
- Conclusion and future work

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Figure: Definition of the strait of Gibraltar (left) and its bathymetry (right).

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SRNH scheme for non linear systems

SRNH scheme for 1D non homogeneous systems of balance laws:

$$\begin{cases} \frac{\partial W}{\partial t} + \frac{\partial F(W)}{\partial x} = Q(x, W) \text{ in } X = D \times]0, T[\qquad (1) \end{cases}$$

$$\begin{cases} W_{i+\frac{1}{2}}^{n} = \frac{1}{2} \left(W_{i+1}^{n} + W_{i}^{n} \right) - \frac{1}{2} \operatorname{sgn} \left[\mathcal{B}_{i+\frac{1}{2}}^{n} \right] \left(W_{i+1}^{n} - W_{i}^{n} \right) \\ + \frac{\Delta x}{2} \left| \mathcal{B}_{i+\frac{1}{2}}^{n} \right|^{-1} Q_{i+\frac{1}{2}}^{n} \tag{2} \\ W_{i}^{n+1} = W_{i}^{n} - r \left(F(W_{i+\frac{1}{2}}^{n}) - F(W_{i-\frac{1}{2}}^{n}) \right) + \Delta t Q_{i}^{n} \end{cases}$$

with : $\mathcal{B}_{i+\frac{1}{2}}^{n} = (\mathcal{R}\Lambda^*\mathcal{R}^{-1})(V(W_i^n, W_{i+1}^n))$ is an approximation of the jacobian matrix calculated at the average state $V(W_i^n, W_{i+1}^n)$.

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$$\begin{cases} \frac{\partial W}{\partial t} + \frac{\partial F(W)}{\partial x} = Q(x, W), \ (x, t) \in \mathcal{D} \times \mathbb{R}_{+}^{\times}, \ \mathcal{D} \subset \mathbb{R} \\ W(x, 0) = W_{0}(x), \ x \in \mathcal{D} \end{cases}$$
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$$W(x,t) = (h(x,t), hu(x,t))^{T}$$

$$F(W(x,t)) = \left(hu(x,t), hu^2(x,t) + \frac{1}{2}gh^2(x,t)\right)^T$$
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$$Q_{i+\frac{1}{2}}^n = -\frac{g}{2\Delta x} \left(h_i^n + h_{i+1}^n \right) \begin{bmatrix} 0\\ z_{i+1} - z_i \end{bmatrix},$$

and

$$Q_{i}^{n} = \frac{1}{\Delta t \Delta x} \int_{t^{n}}^{t^{n+1}} \int_{x_{i-\frac{1}{2}}}^{x_{i+\frac{1}{2}}} Q(x, W(x, t)) dx dt$$

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Definition

W(x, t) is a static stationary solution of the system if $\frac{\partial W}{\partial t} = 0$ and u(x, t) = 0. In this case, one has h(x, t) + z(x) = canstant.

Definition

A finite volume scheme is said to verify the exact C-property if it preserves the equilibrium state:

 $h_i^n + z_i = c$ and $u_i^n = 0$ $\forall (i, n) \in \mathbb{Z} \times \mathbb{N}$.

Proposition

If the source term, in the second step of the scheme, is discretized as follows : $(Q_i^n)_1 = 0$, and i) $(Q_i^n)_2 = -\frac{g}{4\Delta x} \left(h_{i+\frac{1}{2}}^n + h_{i-\frac{1}{2}}^n \right) (z_{i+1} - z_{i-1})$, or ii) $(Q_i^n)_2 = -\frac{g}{8\Delta x} \left(h_{i+1}^n + 2h_i^n + h_{i-1}^n \right) (z_{i+1} - z_{i-1})$ then the scheme (4) respects the exact *C*-property.

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Figure: Bed, free surface and Water momentum t = 10s

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Figure: Water depth at t=0.7s and Error plot (slope $\simeq 0.65$)

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Application of *SRNH* **scheme to non homogeneous 2D Shallow Water flows :**

The system considered may by written as follows :

$$\begin{cases} h_{,t} + (hu)_{,x} + (hv)_{,y} = 0\\ (hu)_{,t} + (hu^2)_{,x} + (huv)_{,y} + g\left(\frac{h^2}{2}\right)_{,x} = -gh(Z_f)_{,x}\\ (hv)_{,t} + (huv)_{,x} + (hv^2)_{,y} + g\left(\frac{h^2}{2}\right)_{,y} = -gh(Z_f)_{,y}, \end{cases}$$
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where *h* is the water level, $\mathbf{u} = {}^{t}(u, v)$ the water velocity and Z_{f} the bottom height.

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To calculate the predictor phase of SRNH scheme, one projects the equations on each interface e_{ij} , and gets the following system (**Abgrall 03**)

$$(U_{\eta})_t + (F_{\eta})_{,\eta} = Q(x, y, U_{\eta})$$
(6)

with

$$U_{\eta} = (h, hu_{\eta}, hu_{\tau})^{T}, \qquad F_{\eta} = \left(hu_{\eta}, hu_{\eta}^{2} + g\frac{h^{2}}{2}, hu_{\eta}u_{\tau}\right)^{T},$$

et $Q(x, y, U_{\eta}) = (0, -gh(Z_{f}), \eta, 0)^{T},$

 $u_{\eta} = \mathbf{u} \cdot \eta, \ u_{\tau} = \mathbf{u} \cdot \tau, \ \eta \text{ and } \tau \text{ the normal and the tangential vector}$ to the interface, and $(.)_{\eta}$ the derivate along the normal vector η .

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For simplicity in presentation we write the equations in a conservative form as:

$$\partial_t \mathbf{W} + \partial_x \left(\mathbf{F}(\mathbf{W}) - \tilde{\mathbf{F}}(\mathbf{W}) \right) + \partial_y \left(\mathbf{G}(\mathbf{W}) - \tilde{\mathbf{G}}(\mathbf{W}) \right) = \mathbf{Q}(\mathbf{W}),$$
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where W and Q are the vectors of conserved variables and source terms, F and G are the convection tensor fluxes, \tilde{F} and \tilde{G} are the diffusion tensor fluxes

$$\mathbf{W} = \begin{pmatrix} h \\ hu \\ hv \\ hC \end{pmatrix}, \quad \mathbf{Q}(\mathbf{W}) = \begin{pmatrix} 0 \\ -gh(S_{0x} + S_{fx}) \\ -gh(S_{0y} + S_{fy}) \\ hQ \end{pmatrix},$$
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$$\mathbf{F}(\mathbf{W}) = \begin{pmatrix} hu \\ hu^2 + \frac{1}{2}gh^2 \\ huv \\ huC \end{pmatrix}, \quad \mathbf{G}(\mathbf{W}) = \begin{pmatrix} hv \\ huv \\ hv^2 + \frac{1}{2}gh^2 \\ + e^{hvC} + e^{hvC} \end{pmatrix},$$

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F

$\widetilde{\mathbf{F}}(\mathbf{W}) = (0, 0, 0, D_{xx}\partial_x (hC) + D_{xy}\partial_y (hC))^T$ $\widetilde{\mathbf{G}}(\mathbf{W}) = (0, 0, 0, D_{yx}\partial_x (hC) + D_{yy}\partial_y (hC))^T$

where D_{xx} , D_{xy} , D_{yx} and D_{yy} are entries of the diffusion matrix **D** assumed to be nonnegative. $S_{0x} = \partial_x Z$, $S_{0y} = \partial_y Z$, with Z(x, y) denotes the bottom topography, while S_{fx} and S_{fy} are the friction losses along the x- and y-direction, and are defined by $S_{fx} = \eta^2 \frac{u\sqrt{u^2 + v^2}}{h^{4/3}}$, $S_{fy} = \eta^2 \frac{v\sqrt{u^2 + v^2}}{h^{4/3}}$, where η is the Manning roughness coefficient.

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Figure: Definition of the strait of Gibraltar (left) and its bathymetry (right).





Figure: Adapted meshes (first row) and pollutant concentration (second row) at different simulation times.



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$$\partial_t h + \partial_x (hu) + \partial_y (hv) = 0,$$

$$\partial_t (hu) + \partial_x \left(hu^2 + \frac{1}{2}gh^2 \right) + \partial_y (huv) = -gh\partial_x Z, \quad (8)$$

$$\partial_t (hv) + \partial_x (huv) + \partial_y \left(hv^2 + \frac{1}{2}gh^2 \right) = -gh\partial_y Z,$$

with the bed-load equation

$$\partial_t Z + \xi \partial_x q_1 + \xi \partial_x q_2 = 0, \tag{9}$$

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with $\xi = rac{1}{1-arepsilon}$ and the sediment transport fluxes are:

$$q_1 = Au \left(\sqrt{u^2 + v^2}\right)^{m-1}, \qquad q_2 = Av \left(\sqrt{u^2 + v^2}\right)^{m-1}, \quad (10)$$

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$$\partial_t (hu_\tau) + \partial_\eta (hu_\eta u_\tau) = 0,$$

$$\partial_t (Z) + \partial_\eta (A\xi(u_\eta^2 + u_\tau^2)) = 0,$$

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where $u_{\eta} = (u, v) \cdot \eta$ and $u_{\tau} = (u, v) \cdot \tau$ are the normal and tangential velocity, respectively.

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the system writes in a non conservative form:

$$rac{\partial U}{\partial t} + \mathcal{A}(U) rac{\partial U}{\partial \eta} = 0,$$

where

$$\mathcal{A}(U) = egin{pmatrix} u_\eta & h & 0 & 0 \ g & u_\eta & 0 & g \ 0 & 0 & u_\eta & 0 \ 0 & A\xi(3u_\eta^2 + u_ au^2) & 2A\xi u_\eta u_ au & 0 \ \end{pmatrix},$$

If we put $\lambda_4 = u_{\eta}$, with $e_4 = [-1, 0, \frac{1}{2A\xi u_{\tau}}, 1]^T$, then the 3 first eigenvalues have the same expression as ine the 1D case, with here $d = A\xi(3u_{\eta}^2 + u_t a u^2)$. Their associated eigenvectors are:

$$e_k = \left(1, \frac{\lambda_k - u_\eta}{h}, 0, \frac{(\lambda_k - u_\eta)^2 - c^2}{c^2}\right)$$

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$$Z(0, x, y) = \begin{cases} \sin^2\left(\frac{(x - 500)\pi}{200}\right) \sin^2\left(\frac{(x - 400)\pi}{200}\right), & \text{if } (x, y) \in \Omega\\ 0, & \text{elsewhere,} \end{cases}$$

where $\Omega = [500, 700] \times [400, 600]$ and the initial water level and initial velocity field are

$$h(0, x, x) = 10 - Z(0, x, y), \quad u(0, x, y) = \frac{Q}{h(0, x, y)}, \quad v(0, x, y) = 0,$$
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where $m = 3$, the porosity $\epsilon = 0.4$ and the discharge $Q = 10 \ m^2/s$.

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Figure: Bed and mesh evolution for the coupled approach using A = 1 at three different times. From left to right t = 50, 300 and 600 s.

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F. Benkhaldoun*, S. Sahmim^x, M. Seaid⁺

Conclusions and future

• Presentation of a new finite volume scheme designed for non homogeneous systems

- The approximate intermediate state is upwind instead of the numerical flux
- Both homogeneous and non homogeneous part of the system are upwind
- Equilibrium for steady states is respected
- New applications were considered (problems of pollutant transport, Shallow water on a moving bed)
- More complex problems (realistic moving bed problems) are under study

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